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6 Bridge Substructure Design

6.1 General

The series of articles under Section 6, Bridge Substructure Design, is intended to fit together as a unit. As much as possible, cross references are used to avoid duplication.

In a following article [BDM 6.1.5], three references are listed: the soils information chart and two integral abutment research reports. Although the soils reference has not been updated for LRFD, it provides background and additional information. Pile design charts in the reference temporarily have been calibrated to LRFD for inclusion in this manual [BDM 6.2.7], and three research projects are being conducted by lowa State University to calibrate pile design based on test data, reliability theory, and construction control.

In the past designers were encouraged to obtain and use the 1987 integral abutment reference for relatively long bridges; however, the office no longer encourages use of the reference because the bridge length policy recently has been revised to allow longer bridges, and the 2005 reference has modified the older recommended design procedures [BDM Table 6.5.1.1.1]. The two integral abutment references generally substantiate the present policy, and the designer may find them useful in understanding the various rules associated with the policy.

6.1.1 Policy overview

In the Office of Bridges and Structures, the design of typical highway bridge substructures proceeds from preliminary to final design sections. The preliminary design section selects the abutment, pier, and foundation type based on bridge site information and criteria stated in *Bridge Design Manual* Section 3, Preliminary Design. In some cases the preliminary design section also considers aesthetic criteria in Section 4, Aesthetic Design (in process). Final design sections then complete the structural design and detailing following policies in Section 5, Bridge Superstructure Design, and Section 6, Bridge Substructure Design.

For a bridge over a waterway a Corps of Engineers 404 permit is required, but there no longer is a need for a note on the bridge plans. For typical bridge projects the Office of Design will provide notes and details for the 404 permit. If special permit conditions are identified in the submittal letter from the Office of Location and Environment (Green Sheet), the Office of Bridges and Structures will work with the Office of Design to provide appropriate notes and details in the project plans.

In the past the office has followed the AASHTO Standard Specifications but now is in transition to the AASHTO LRFD Specifications. The office interprets the basic LRFD specifications when designing foundations, abutments, and piers and specifies rules for detailing of those substructure components. This series of articles on substructure components covers most typical designs but does not cover special designs for signature bridges and long-span bridges.

In all cases substructure components need to be designed for vertical and lateral loads, settlement, stability, and economy considering the complete bridge structure. Pile, drilled shaft, footing, abutment, and pier design is affected by the choice of bearings [BDM 5.7] because bearings transmit loads from the superstructure. Abutment and pier design also is affected by the choice of foundations because foundations provide support and either partial or complete fixity.

For highway bridge foundations, the office generally selects among three types: piles, drilled shafts, and spread footings. Because most lowa bridge sites are in rural areas and because bedrock seldom is near the surface, site conditions and economy usually favor the use of piles for the support of substructure components. In cases where pile driving would disturb adjacent structures and where site conditions permit, substructure components may be supported on drilled shafts. In cases where bedrock is close to the planned bottom elevation of a substructure component, the component should be placed on a spread footing notched into the rock.

In order to eliminate expansion joints, wherever practical the office selects integral abutments instead of other abutment types. Based on research and testing for bridges without skew, the office generally orients integral abutment steel H-piles for weak axis bending with respect to longitudinal expansion or contraction of the superstructure. For bridges with skews of 30 degrees or less, H-piles are rotated to align the pile webs with the centerline of abutment bearings. However, at skews above 30 degrees, piles shall be aligned with pile webs perpendicular to centerline of roadway.

Except for integral abutments, wherever practical the office batters some of the foundation piles for a substructure component. Typically the inner line of stub abutment piles, stub abutment wing wall piles, and the perimeter piles for pier foundations are battered at a 1 horizontal to 4 vertical slope. The end piles for pile bents are battered at a 1 horizontal to 12 vertical slope.

6.1.2 Design information

For each bridge site the Soils Design Section in the Office of Design provides to the Office of Bridges and Structures a bridge soils package that typically includes three primary items: the boring logs, the soil profile sheet, and the supplemental. There may be other items or attachments as needed based on site conditions and other factors.

The supplemental is organized to provide the N-values for the standard penetration test, but the document typically provides additional information, requirements, and recommendations for three design factors: slope stability, settlement, and foundation. These three items can vary from very short to complex, can discuss work to be done by a grading contractor, and can discuss further coordination between design offices.

For the foundation part of the supplemental, the Soils Design Section typically recommends a foundation type, usually one of the following:

- Point-bearing piles driven to a rock formation,
- Friction or friction plus bearing piles driven to a specified load capacity, below any expected scour elevation,
- · Drilled shafts, or
- Spread footings founded directly on a rock formation.

The Soils Design Section does not necessarily make more specific recommendations, such as a pile type. If applicable, the section discusses downdrag and relates downdrag to design requirements and delay period. The settlement information on the supplemental includes consolidation time rate information.

The Soils Design Section typically recommends pile driving points if soil conditions warrant, but the designer may discuss the need with the section if it appears that points are advisable.

The majority of Iowa bridge foundations are placed on piles that derive support from shear strength of surrounding soil and end bearing. Based on approximately 280 pile load tests in Iowa soils and

experience with previous charts the Soils Design Section prepared *Foundation Soils Information Chart, Pile Foundation* in 1989, updated the chart in 1994, and accepted charts updated to LRFD geotechnical resistance for trial use in 2006. The revised 2007 LRFD geotechnical resistance charts [BDM 6.2.7] shall be used for design of pile foundations for lowa bridges.

6.1.3 Definitions

Substructure is any construction below the bearing seats or, in the absence of bearings, below the soffit of the superstructure.

6.1.4 Abbreviations and notation

N or N-value, standard penetration test number of blows per foot (300 mm). N also may be given as SPT NO, the Standard Penetration Test Number, in the soils information chart reference.

6.1.5 References

Abendroth, R.E. and L.F. Greimann. *Field Testing of Integral Abutments, Final Report HR-399.* Ames: Center for Transportation Research and Education, Iowa State University, 2005. (Available on the Internet at http://www.iowadot.gov/operationsresearch/reports/reports_pdf/hr_and_tr/reports/hr399.pdf)

Dirks, Kermit and Patrick Kam. *Foundation Soils Information Chart, Pile Foundation*. Ames: Iowa Department of Transportation, Office of Road Design, January 1989/September 1994. (Contact the Soils Design Section in the Office of Design for a copy of the publication. For LRFD, use the updated geotechnical resistance charts in this manual [BDM 6.2.7].)

Greimann, L.F., R.E. Abendroth, D.E. Johnson, and P.B. Ebner. *Final Report, Pile Design and Tests for Integral Abutment Bridges, HR-273, and Addendum.* Ames: Iowa Department of Transportation and College of Engineering, Iowa State University, 1987. (Available on the Internet athttp://www.iowadot.gov/operationsresearch/reports/reports_pdf/hr_and_tr/reports/hr273.pdf)